



Beach Profiles

AND CROSS-SHORE TRANSPORT

CE A676 Coastal Engineering
Orson P. Smith, PE, Ph.D., Professor Emeritus



Topics

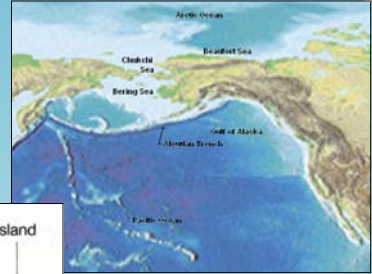
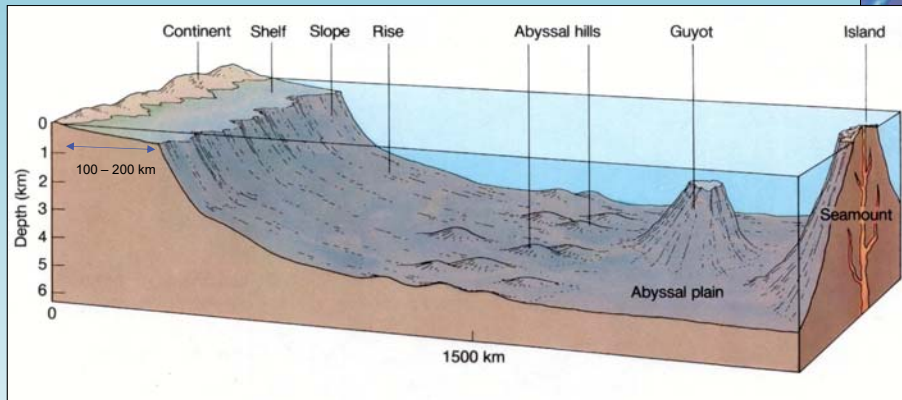
- Features of beach and nearshore profiles
- Equilibrium profiles
- Cross-shore transport

References

- Text (Sorensen) Ch. 8
- CEM Part III Ch. 3
- Komar, P., 1998. Beach Processes and Sedimentation, 2nd ed., Prentice-Hall



The Big Picture



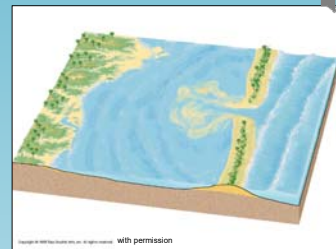
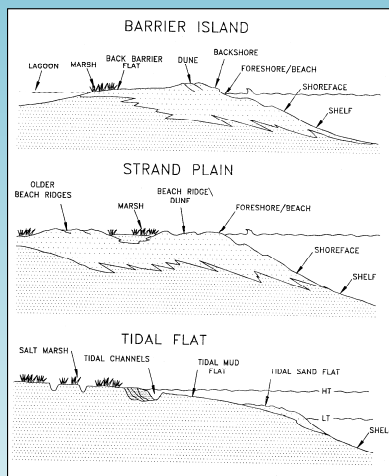
BEACH PROFILES AND CROSS-SHORE TRANSPORT

CE A676 COASTAL ENGINEERING - UAA COLLEGE OF ENGINEERING

3

Shore types

- Barrier islands
 - Separated by tidal inlets
 - Dynamic equilibrium
 - Prone to dramatic change by severe storms
- Strand plain
 - Similar to barrier island profile
- Tidal flat
 - Higher tide range

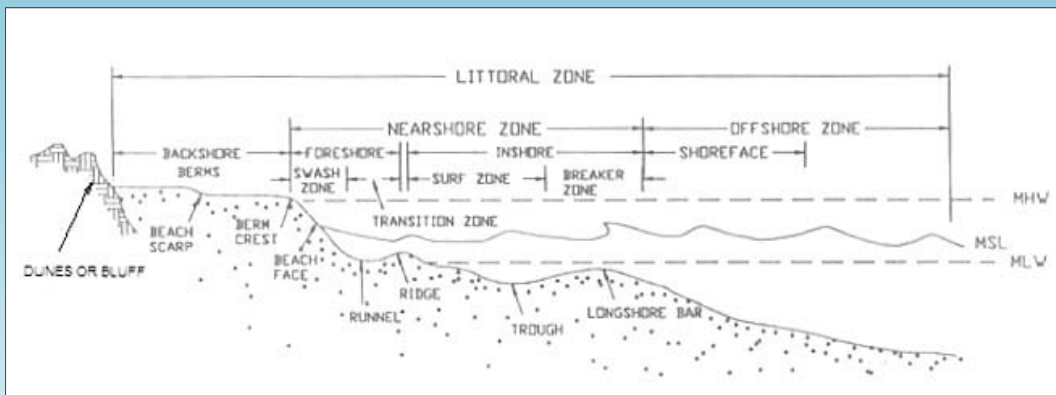


BEACH PROFILES AND CROSS-SHORE TRANSPORT

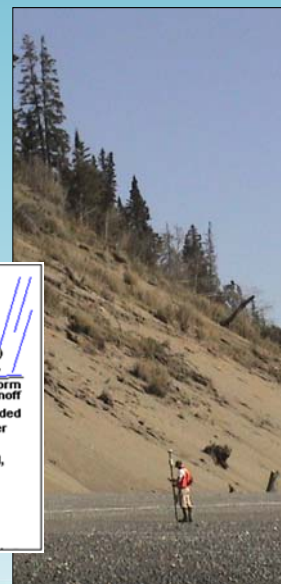
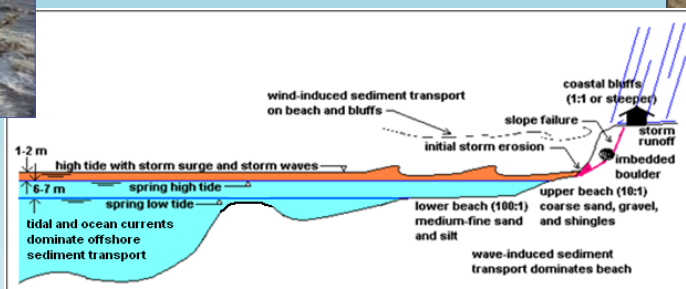
CE A676 COASTAL ENGINEERING - UAA COLLEGE OF ENGINEERING

4

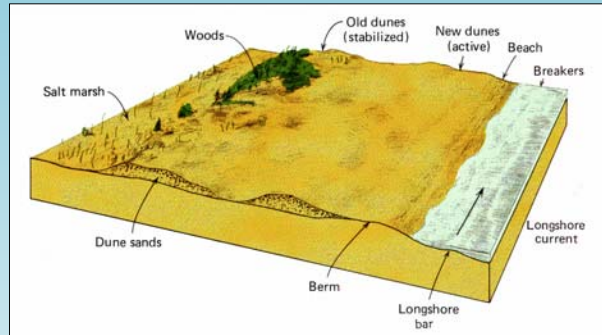
Beach Profile Features



Cook Inlet bluff shoreline



Dunes



BEACH PROFILES AND CROSS-SHORE TRANSPORT

CE A676 COASTAL ENGINEERING - UAA COLLEGE OF ENGINEERING

7

Equilibrium Profile

- Profile of a beach in balance

- *Source = Supply* for longshore and cross-shore sediment transport

- $h = Ax^n$

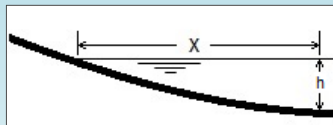
- $n = 2/3$, A has units $m^{1/3}$ (Per Bruun, 1954)

- $A = 0.21D^{0.48}$, $D = D_{50}$ in mm (R. Dean, 1987)

- Applications

- Compare actual profile to ideal

- Steeper → erosion
 - Shallower → accretion
 - Beach fill design



- Issues

- No tide allowance

- No Berms

- No $x < 0$

- Problem with slope ($dh/dx = \infty$ at $x = 0$)

- Can't use too near waterline

- No depth of closure (limit of wave-induced sediment transport)

- $h_c \cong 6.75H_s$

- H_s = an average (seasonal or annual) significant wave height

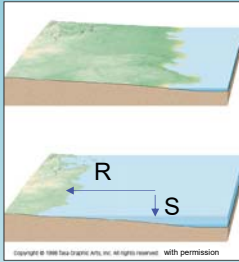
- Don't use for $z > h_c$

BEACH PROFILES AND CROSS-SHORE TRANSPORT

CE A676 COASTAL ENGINEERING - UAA COLLEGE OF ENGINEERING

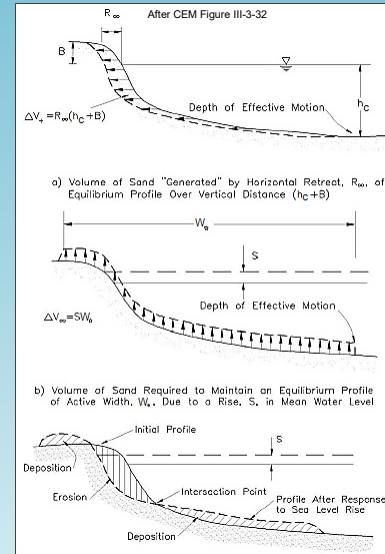
8

Sea Level Rise – Bruun Rule



$$R_{\infty} = S \frac{W_*}{h_c + B}$$

- R_{∞} = static response horizontal shoreline retreat of equilibrium profile
- S = vertical sea level rise
- W_* = width of active profile
 - $W_* = \left(\frac{h_c}{A}\right)^{3/2}$ CEM Eqn. III-3-19
- $h_c + B$ = vertical extent of active profile
- h_c = closure depth
- $R(h_c + B)$ = sediment lost from profile due to sea level rise



Bruun Rule – Sea Level Rise Example

- After CEM Example Problem III-3-5:
 - $h_c = 6$ m, $B = 2$ m, $D = 0.2$ mm, $dS/dt = 3$ mm/yr
 - What is rate of shoreline retreat?
 - What rate of beach fill is required to prevent retreat?

$$A = 0.21D^{0.48} = 0.1m^{1/3} \quad W_* = \left(\frac{h_c}{A}\right)^{3/2} = 465 \text{ m}$$

$$R_{\infty} = S \frac{W_*}{h_c + B} \quad \frac{dR_{\infty}}{dt} = \frac{dS}{dt} \left(\frac{W_*}{h_c + B}\right) \cong 0.17 \text{ m/yr}$$

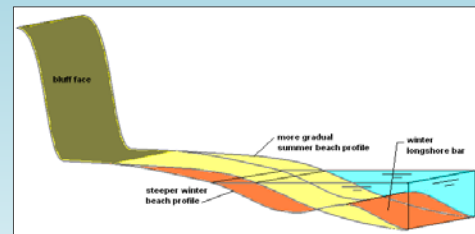
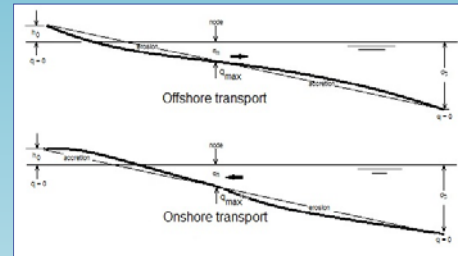
$$\frac{dV}{dt} = W_* \frac{dS}{dt} \cong 1.4 \frac{m^3}{m} / \text{yr}$$

Slope at the beach face (shoreline)

- Variety of empirical formulae in literature
 - Response to Bruun/Dean equilibrium profile problem at $x = 0$
 - For example: $S_0 = 0.25 \left(\frac{D}{H_o}\right)^{0.25} \left(\frac{H_o}{L_o}\right)^{-0.15}$
 - S_0 = slope at the shoreline
 - $D = D_{50}$
 - H_o = deep water wave height
 - L_o = deep water wave length
 - Can be applied to a modified Bruun/Dean derivative relationship:
 - $$S = \frac{S_0}{1 + \frac{3}{2} \left(\frac{S_0}{A^{3/2}}\right) h^{1/2}}$$
 - S = slope at depth h

Cross-shore change

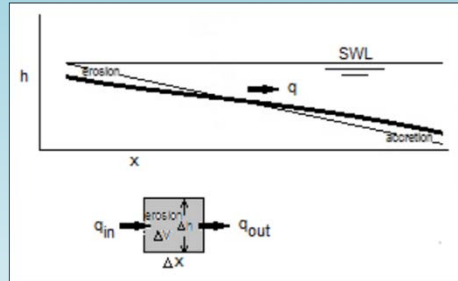
- Wave induced sediment transport extends from
 - limit of runup onshore
 - to limit of wave-induced motion offshore (depth of closure)
- Maximum transport in the surf zone is at the node between erosion and accretion
- Winter offshore bar profile is associated with net *offshore* transport
 - High wave conditions
 - Steeper beach face
- Summer “berm-type” profile is associated with net *onshore* transport
 - Low wave conditions
 - Shallower beach face slope



Cross-shore transport prediction

SBEACH model formulation

- Elevation change Δh across Δx (cross-shore increment)
- Uniform conditions in Δy (longshore increment)
- Leads to volume change ΔV
- Sediment transport $q = \frac{dV}{dt}$
- $\frac{\Delta q}{\Delta x} = -\frac{\Delta h}{\Delta t} \rightarrow \frac{dq}{dx} = -\frac{dh}{dt}$



SBEACH cross-shore zones

- Zone I
 - $q = q_b e^{-\gamma x}$
 - q_b = rate at break point
 - x coordinate from break point
 - Spatial decay coefficient
 - $\gamma = 10.3 \left(\frac{D}{H_b}\right)^{0.47}$
 - D = grain size
 - H_b = breaking wave height
- Zone II: $q = q_b e^{-\gamma x}$, $\gamma=0.21$
- Zone III and IV require calibration measurements (field, physical model, or both)

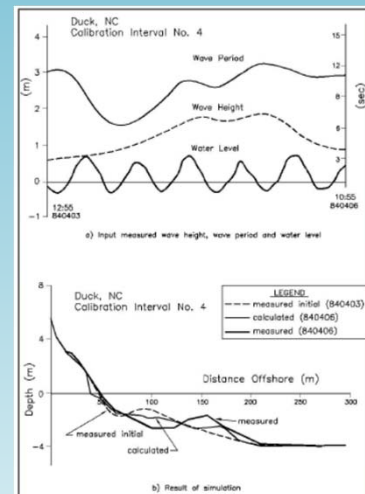
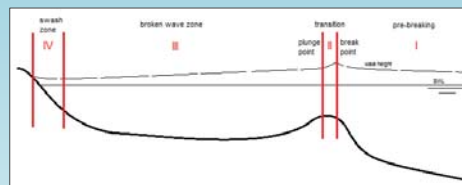


Figure 8.3.44. SBEACH tested against profile evolution data from Duck, NC (Larson and Kraus 1988) Coastal Engineering Manual Part III Ch. 3

